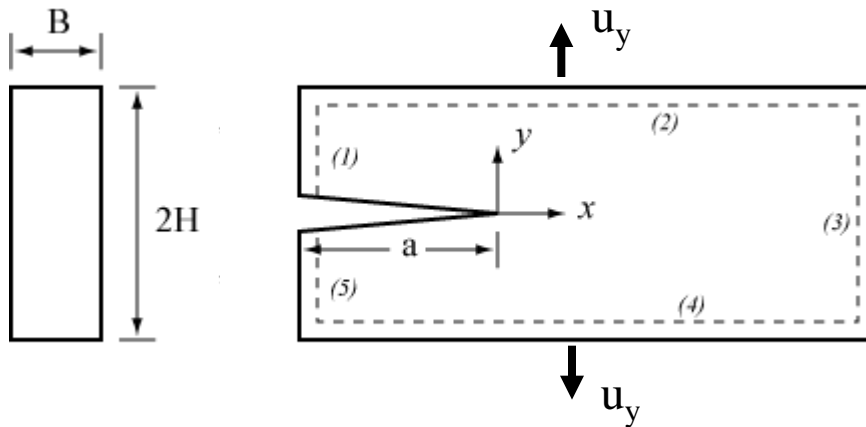




دانشگاه صنعتی اصفهان
دانشکده مکانیک

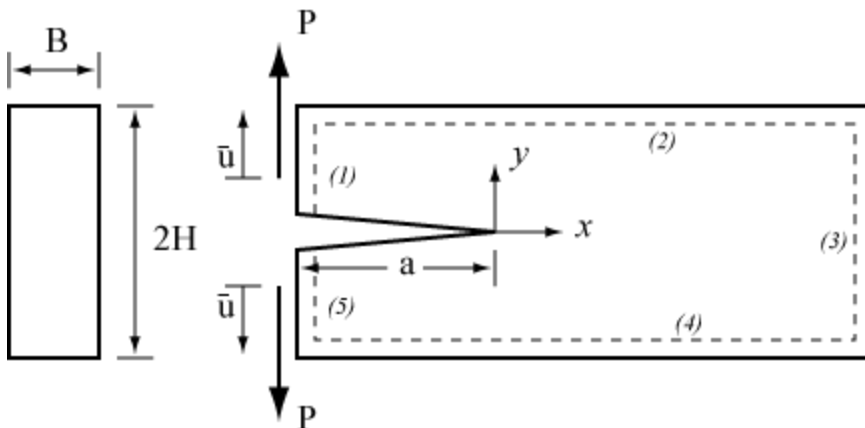
The J integral

Example 1



$$J = 2hw = \frac{(1-\nu)Eu_y^2}{(1+\nu)(1-2\nu)h}$$

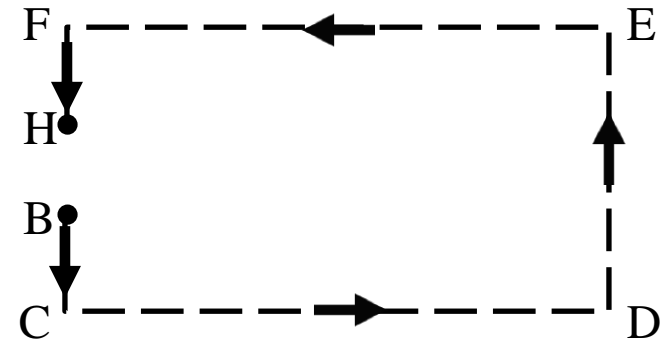
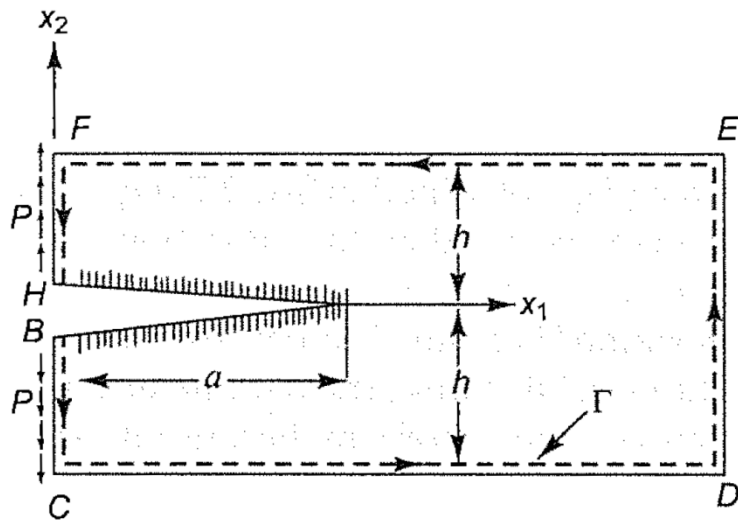
Example 2



$$J = \frac{12P^2 a^2}{EB^2 h^3}$$

Example 3:

J integral for double cantilever beam, if each cantilever is pulled by a distributed load P , as shown



The chosen path Γ is BCDEFH and it coincides with the body contour.

Contour of the crack; therefore, $J = J_{BC} + J_{CD} + J_{DE} + J_{EF} + J_{FH}$

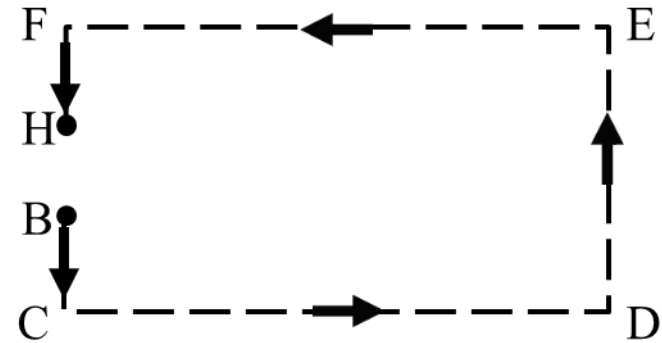
J_{BC} : As bending moment is zero, bending stress is zero. So, $w=0$

$$J_{BC} = \int_{BC} \left(w \, dy - T_i \frac{\partial u_i}{\partial x} ds \right)$$

$$= \int_{BC} \left(0 - T_i \frac{\partial u_i}{\partial x} ds \right)$$

$$= - \int_{BC} T_i \frac{\partial u_i}{\partial x} ds$$

$$J_{BC} = - \int_0^h T \frac{\partial v}{\partial x} dy$$



Assuming:

- A small element of length dy on the path then $ds = dy$
- Along y-direction $u_i = v$
- Length is h so limits are 0 to h .

Contour of the crack; therefore, $J = J_{BC} + J_{CD} + J_{DE} + J_{EF} + J_{FH}$

$$J_{CD}: \quad J_{CD} = \int_{CD} \left(w \, dy - T_i \frac{\partial u_i}{\partial x} ds \right)$$

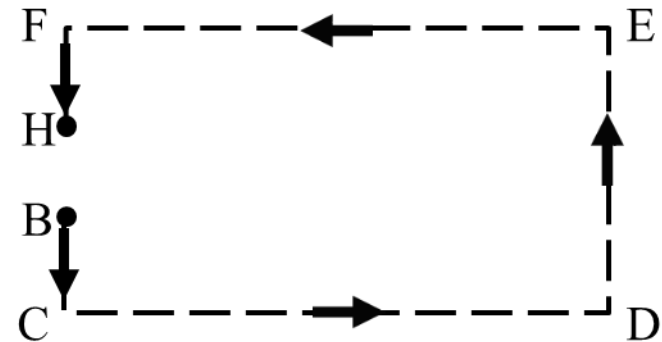
dy is negligible and $T_i = 0 \Rightarrow J_{CD} = 0$

$$J_{EF}: \quad J_{EF} = \int_{EF} \left(w \, dy - T_i \frac{\partial u_i}{\partial x} ds \right)$$

dy is negligible and $T_i = 0 \Rightarrow J_{EF} = 0$

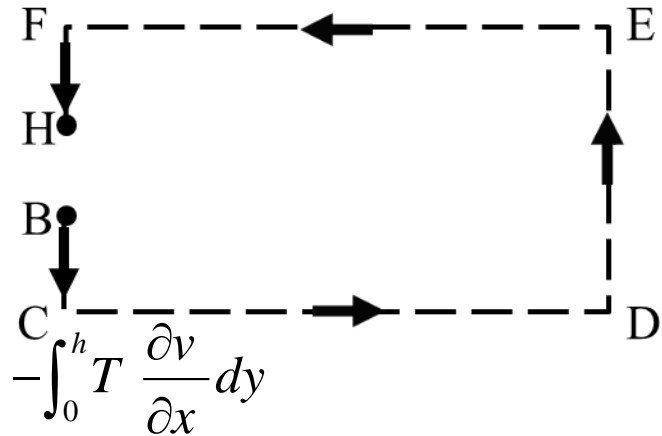
$$J_{DE}: \quad J_{DE} = \int_{DE} \left(w \, dy - T_i \frac{\partial u_i}{\partial x} ds \right)$$

stresses are very small, which in turn, make w and T_i negligible. $\Rightarrow J_{DE} = 0$



Contour of the crack; therefore, $J = J_{BC} + J_{CD} + J_{DE} + J_{EF} + J_{FH}$

$$J_{FH} = \int_{FH} \left(w \, dy - T_i \frac{\partial u_i}{\partial x} \, ds \right)$$



On segments BC and FH, w is negligible, \Rightarrow

$$J_{FH} = - \int_0^h T \frac{\partial v}{\partial x} \, dy$$

Hence $J = J_{BC} + J_{CD} + J_{DE} + J_{EF} + J_{FH}$

$$\begin{aligned} J &= - \int_0^h T \frac{\partial v}{\partial x} \, dy + 0 + 0 + 0 - \int_0^h T \frac{\partial v}{\partial x} \, dy \\ &= -2 \int_0^h T \frac{\partial v}{\partial x} \, dy \end{aligned}$$

Now we can find $\frac{\partial v}{\partial x}$ using the bending moment equation; Bending moment = $P \cdot x$

$$\frac{\partial^2 v}{\partial x^2} = \frac{Px}{EI} \Rightarrow \frac{\partial v}{\partial x} = \frac{P}{EI} \frac{x^2}{2} + c \quad \left(\text{at } x=a, \frac{\partial v}{\partial x} = 0 \right) \Rightarrow c = -\frac{P}{EI} \frac{a^2}{2} \Rightarrow \frac{\partial v}{\partial x} = \frac{P}{EI} \frac{x^2}{2} - \frac{P}{EI} \frac{a^2}{2}$$

Contour of the crack; therefore, $J = J_{BC} + J_{CD} + J_{DE} + J_{EF} + J_{FH}$

$$\left(\text{at } x=0, \frac{\partial v}{\partial x} = -\frac{P}{EI} \frac{a^2}{2} \right) \quad \text{and} \quad I = \frac{Bh^3}{12}$$

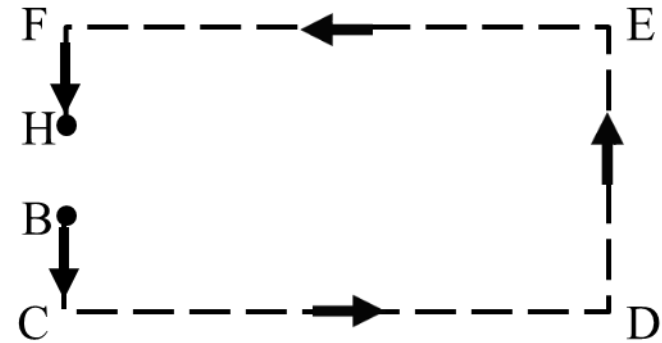
$$\Rightarrow \frac{\partial v}{\partial x} = -\frac{6Pa^2}{EBh^3}$$

Hence

$$J = -2 \int_0^h T * \left(-\frac{6Pa^2}{EBh^3} \right) dy$$

$$= \frac{12Pa^2}{EBh^3} \int_0^h T dy$$

But on face FH: $B \int_0^h T dy = P \quad \Rightarrow \quad J = \frac{12P^2 a^2}{EB^2 h^3}$



Nonlinear Energy Release Rate

- Similar to linear elastic materials, the energy release rate defines for nonlinear elastic materials, except that G is replaced by J :

$$J = -\frac{d\Pi}{dA}$$

The Π is the potential energy and A is crack area: $\Pi = U - F$

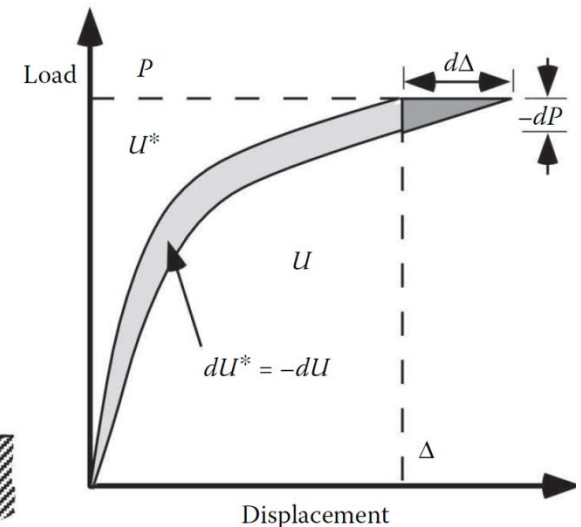
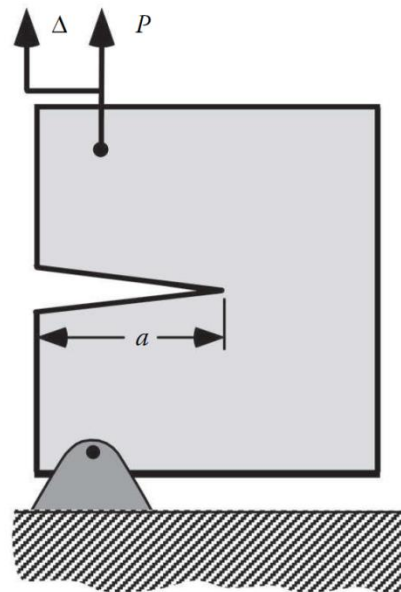
U is the strain energy stored in the body and F is the work done by external forces.

Consider a cracked plate which exhibits a nonlinear load–displacement curve, If the plate has unit thickness, $A = a$,

For load control: $\Pi = U - P\Delta = -U^*$

The U^* is the complementary strain energy,:

$$U^* = \int_0^P \Delta dP$$



Nonlinear Energy Release Rate

In the load control:

$$J = \left(\frac{dU^*}{da} \right)_P$$

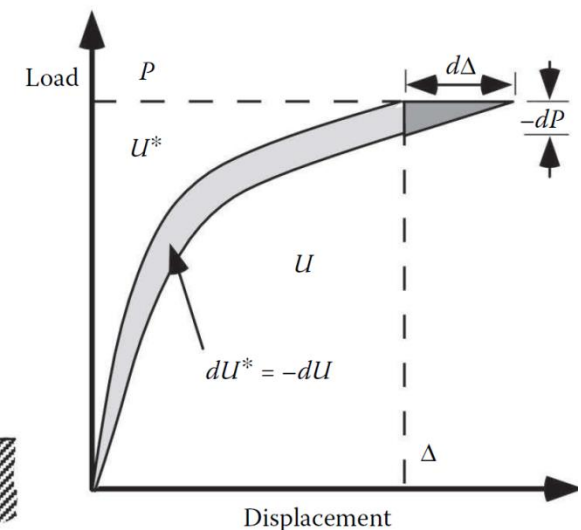
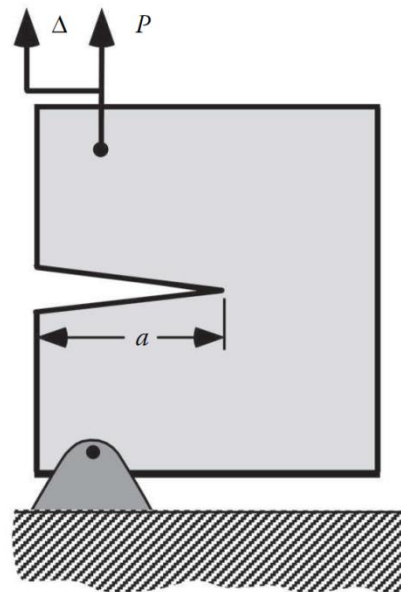
If the crack advances at a fixed displacement, $F=0$, and J is given by: $J = - \left(\frac{dU}{da} \right)_\Delta$

dU^* for load control differs from $-dU$ for displacement control by the amount $\frac{1}{2}dP\Delta$, which is vanishingly small compared to dU . Therefore, J for load control is equal to J for displacement control.

➤ The J in terms of load and displacement:

$$J = \left(\frac{\partial}{\partial a} \int_0^P \Delta dP \right)_P = \int_0^P \left(\frac{\partial \Delta}{\partial a} \right)_P dP$$

$$J = - \left(\frac{\partial}{\partial a} \int_0^\Delta P d\Delta \right)_\Delta = - \int_0^\Delta \left(\frac{\partial P}{\partial a} \right)_\Delta d\Delta$$

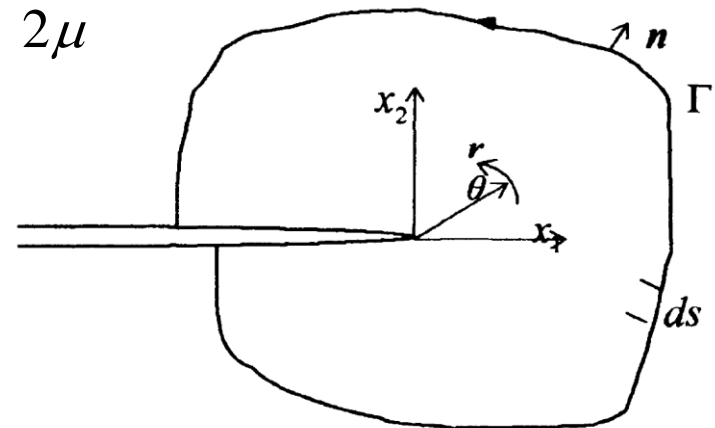


Example 4:

Write the general expression for the J-integral for anti-plane shear, i.e. mode III. Using the dominant elastic singularity, shrink the J-contour down to the crack tip and prove, by direct calculation, that

$$J = \frac{K_{III}^2}{2\mu}$$

$$J = \int_{\Gamma} \left(w n_1 - T_i \frac{\partial u_i}{\partial x_1} \right) ds$$



Path for the calculation of J-integral

where Γ is the path along which the J-integral is calculated, $w = \sigma_{ij}\varepsilon_{ij}/2$ is the strain energy density for linear elastic material, n_1 is the component of the outward unit normal \mathbf{n} in the x_1 -direction, $T_i = \sigma_{ij}n_j$ and \mathbf{u}_i are, respectively, the traction and displacement vectors along Γ and ds is differential arc length.

Example 4:

For a crack under conditions of mode III deformation:

$$\begin{Bmatrix} \sigma_{13} \\ \sigma_{23} \end{Bmatrix} = \frac{K_{III}}{\sqrt{2\pi r}} \begin{Bmatrix} -\sin \frac{\theta}{2} \\ \cos \frac{\theta}{2} \end{Bmatrix}, \quad u_3 = \frac{2K_{III}}{\mu} \sqrt{\frac{r}{2\pi}} \sin \frac{\theta}{2}$$

For mode III deformation, the strain energy density is:

$$w = \sigma_{13}\varepsilon_{13} + \sigma_{23}\varepsilon_{23} = \sigma_{13} \frac{\sigma_{13}}{2\mu} + \sigma_{23} \frac{\sigma_{23}}{2\mu} = \frac{1}{2\mu} (\sigma_{13}^2 + \sigma_{23}^2) = \frac{1}{2\mu} \left(\frac{K_{III}^2}{2\pi r} \right) = \frac{K_{III}^2}{4\pi\mu r}$$

Traction: $T_1 = T_2 = 0$

$$\begin{aligned} T_3 &= \sigma_{31}n_1 + \sigma_{32}n_2 = \sigma_{31} \cos \theta + \sigma_{32} \sin \theta \\ &= \frac{K_{III}}{\sqrt{2\pi r}} \left(-\sin \frac{\theta}{2} \cos \theta + \cos \frac{\theta}{2} \sin \theta \right) = \frac{K_{III}}{\sqrt{2\pi r}} \sin \frac{\theta}{2} \end{aligned}$$

Example 4:

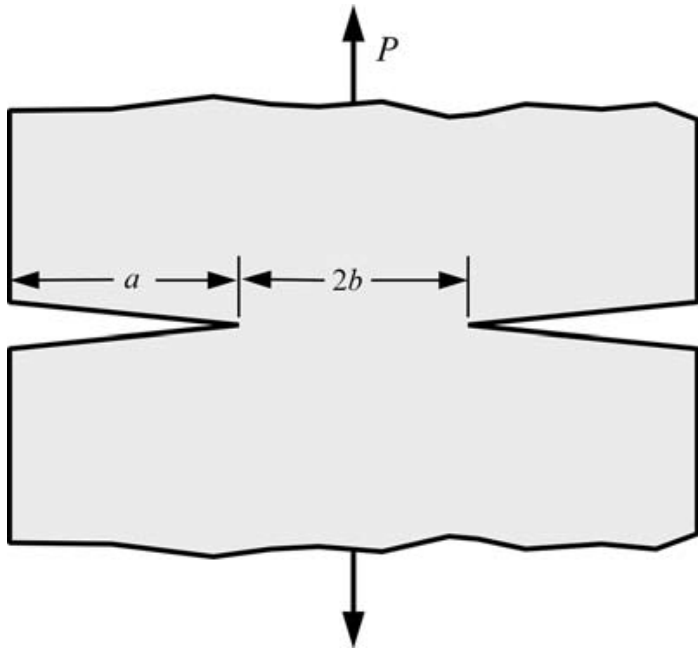
Displacement Derivative: Since the displacement u_3 is a function of the polar coordinates r and θ

$$\begin{aligned} \frac{\partial u_3}{\partial x_1} &= \frac{\partial u_3}{\partial r} \frac{\partial r}{\partial x_1} + \frac{\partial u_3}{\partial \theta} \frac{\partial \theta}{\partial x_1} = \left(\frac{K_{III}}{\mu \sqrt{2\pi r}} \sin \frac{\theta}{2} \right) (\cos \theta) + \left(\frac{K_{III}}{\mu} \sqrt{\frac{r}{2\pi}} \cos \frac{\theta}{2} \right) \left(-\frac{1}{r} \sin \theta \right) \\ &= -\frac{K_{III}}{\mu \sqrt{2\pi r}} \sin \frac{\theta}{2} \end{aligned}$$

The J-integral for mode III deformation is:

$$J = \frac{K_{III}^2}{2\pi\mu r} \int_{-\pi}^{\pi} \left(\frac{1}{2} \cos \theta + \sin^2 \frac{\theta}{2} \right) r d\theta = \frac{K_{III}^2}{2\pi\mu r} \left(\frac{1}{2} \theta \Big|_{-\pi}^{\pi} \right) (r) = \frac{K_{III}^2}{2\pi\mu r} \frac{1}{2} 2\pi r = \frac{K_{III}^2}{2\mu}$$

Double-edge-notched tension (DENT)



Assume that b is the only length dimension that influences Δ_p .

$$\Delta_p = bH\left(\frac{P}{b}\right) \Rightarrow \left. \begin{aligned} \left(\frac{\partial \Delta_p}{\partial b}\right)_P &= H\left(\frac{P}{b}\right) - H'\left(\frac{P}{b}\right)\frac{P}{b} \\ \left(\frac{\partial \Delta_p}{\partial P}\right)_b &= H'\left(\frac{P}{b}\right) \end{aligned} \right\} \left(\frac{\partial \Delta_p}{\partial b}\right)_P = \frac{1}{b} \left[\Delta_p - P \left(\frac{\partial \Delta_p}{\partial P}\right)_b \right]$$

$$J = \frac{1}{2} \int_0^P \left(\frac{\partial \Delta}{\partial a}\right)_P dP = -\frac{1}{2} \int_0^P \left(\frac{\partial \Delta}{\partial b}\right)_P dP$$

$$\Delta = \Delta_{el} + \Delta_p$$

$$J = -\frac{1}{2} \int_0^P \left[\left(\frac{\partial \Delta_{el}}{\partial b}\right)_P + \left(\frac{\partial \Delta_p}{\partial b}\right)_P \right] dP$$

$$= \frac{K_I^2}{E'} - \frac{1}{2} \int_0^P \left(\frac{\partial \Delta_p}{\partial b}\right)_P dP$$

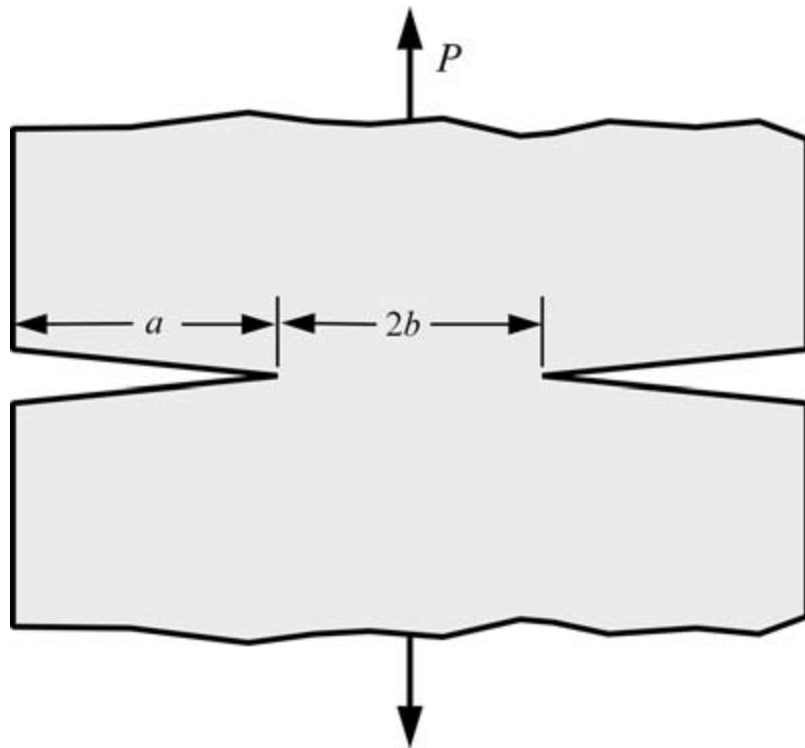
For plane stress

$$E' = E$$

For plane strain

$$E' = E/(1-\nu^2)$$

Double-edge-notched tension (DENT)



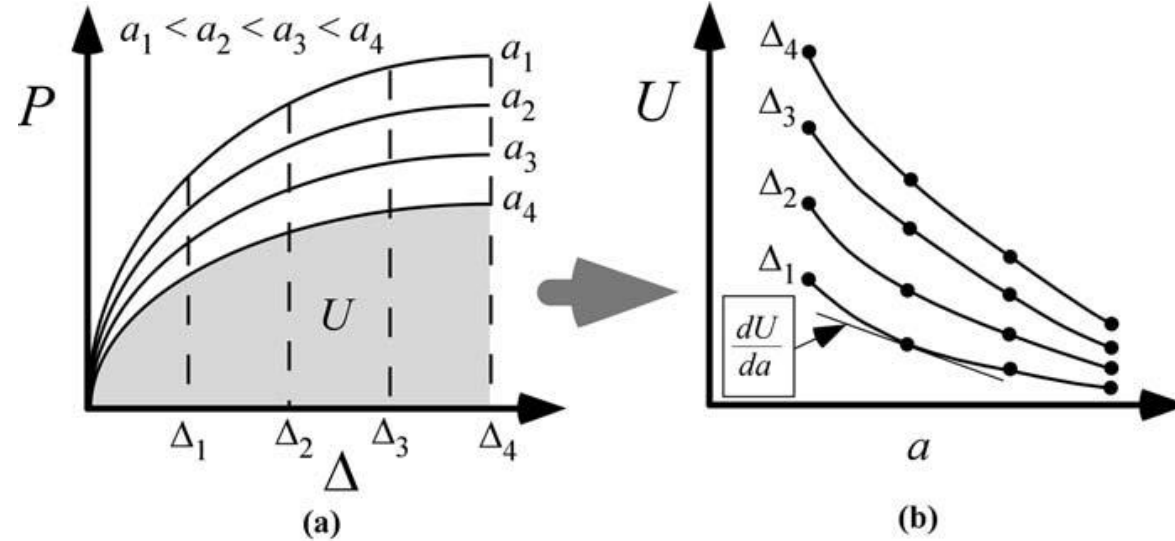
$$\left(\frac{\partial \Delta_p}{\partial b} \right)_P = \frac{1}{b} \left[\Delta_p - P \left(\frac{\partial \Delta_p}{\partial P} \right)_b \right]$$

$$J = -\frac{1}{2} \int_0^P \left[\left(\frac{\partial \Delta_{el}}{\partial b} \right)_P + \left(\frac{\partial \Delta_p}{\partial b} \right)_P \right] dP$$

$$= \frac{K_I^2}{E'} - \frac{1}{2} \int_0^P \left(\frac{\partial \Delta_p}{\partial b} \right)_P dP$$

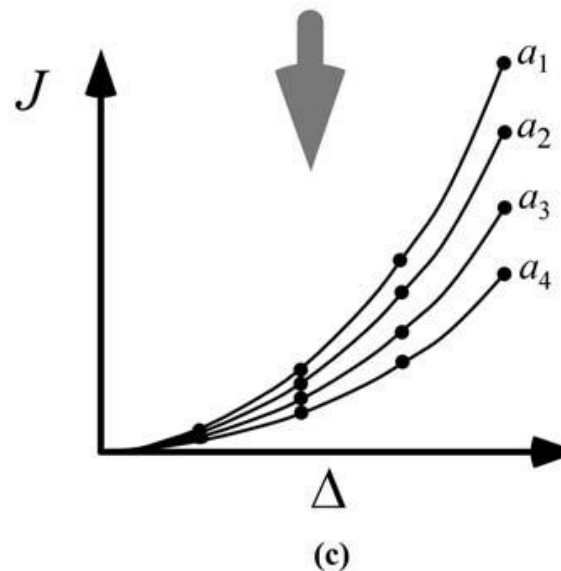
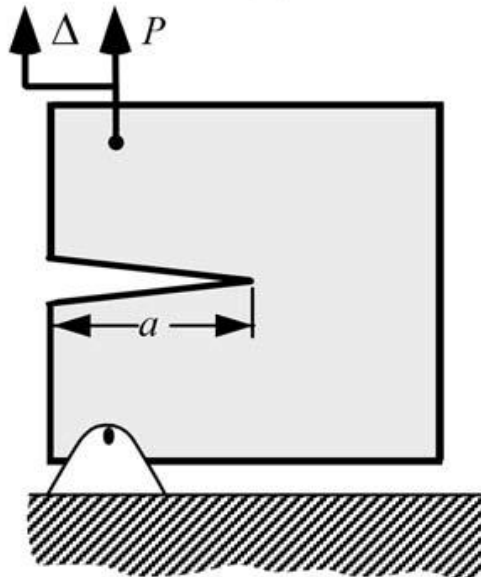
$$J = \frac{K_I^2}{E'} + \frac{1}{2b} \left[2 \int_0^{\Delta_p} P d\Delta_p - P \Delta_p \right]$$

Laboratory Measurement



Computing the J integral is somewhat difficult when the material is nonlinear

U vs. crack length at various fixed displacements



$$J = -\frac{1}{B} \left(\frac{\partial U}{\partial a} \right)_{\Delta}$$